



Prepared for:

## Dink and Dine

Project Address:  
1017 N Dobson Rd & 1065 N Dobson Rd  
Mesa, AZ  
85201

Legal Description & Project Data:  
Assessor Parcel: 135-33-545  
Zoning: LC  
Occupancy Type: A-2  
Addt'l Occupancy: B & S  
Construction Type: III-B

From: Kasha Burlington

cell: 317.366.4594

email: kburlington@ad-s.com

From: Brittney Lawton

cell: 714.469.8898

email: blawton@ad-s.com

### Scope of Work:

| PAGE NO | SIGN ID | SIGN TYPE    | D/F (Y or N) | ILLUM'D (Y or N) | SIGN DIMENSIONS            | SIGN QTY | SIGN AREA   |
|---------|---------|--------------|--------------|------------------|----------------------------|----------|-------------|
| IC.00   | IC-101  | CHNL LTRS    | N            | Y                | 29'-6" x 2'-3"             | 1        | 66.4 sq ft  |
| STL.00  | STL-101 | CHNL LTRS    | N            | Y                | 21'-3" OAL x 6'-5 3/4" OAH | 1        | 121.3 sq ft |
| TL.00   | TL-101  | ARCH GRAPHIC | N            | N                | 6'-4 1/2" x 19'-5 1/2"     | 1        | 76.5 sq ft  |
| BE.00   | BE-101  | CHNL LTRS    | N            | Y                | 16'-5" x 3'-0"             | 1        | 49.3 sq ft  |
| ETL.00  | ETL-101 | CHNL LTRS    | N            | Y                | 21'-3" OAL x 6'-5 3/4" OAH | 1        | 121.3 sq ft |

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If you would like to see a sample of any color, please request an actual sample from your Salesperson or Project Manager.  
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Client Name (Please Print) \_\_\_\_\_ Date \_\_\_\_/\_\_\_\_/\_\_\_\_

Client Name (Signature) \_\_\_\_\_

PLEASE INITIAL ALL PAGES

AD/S Drawing #36984 Rev. #06 (PS) Rev. Date: 04-30-25

Pages: 19 (Including Cover Sheet)

☐ Approved

☐ Approved as Noted

☐ Revise & Resubmit

☐ Rejected

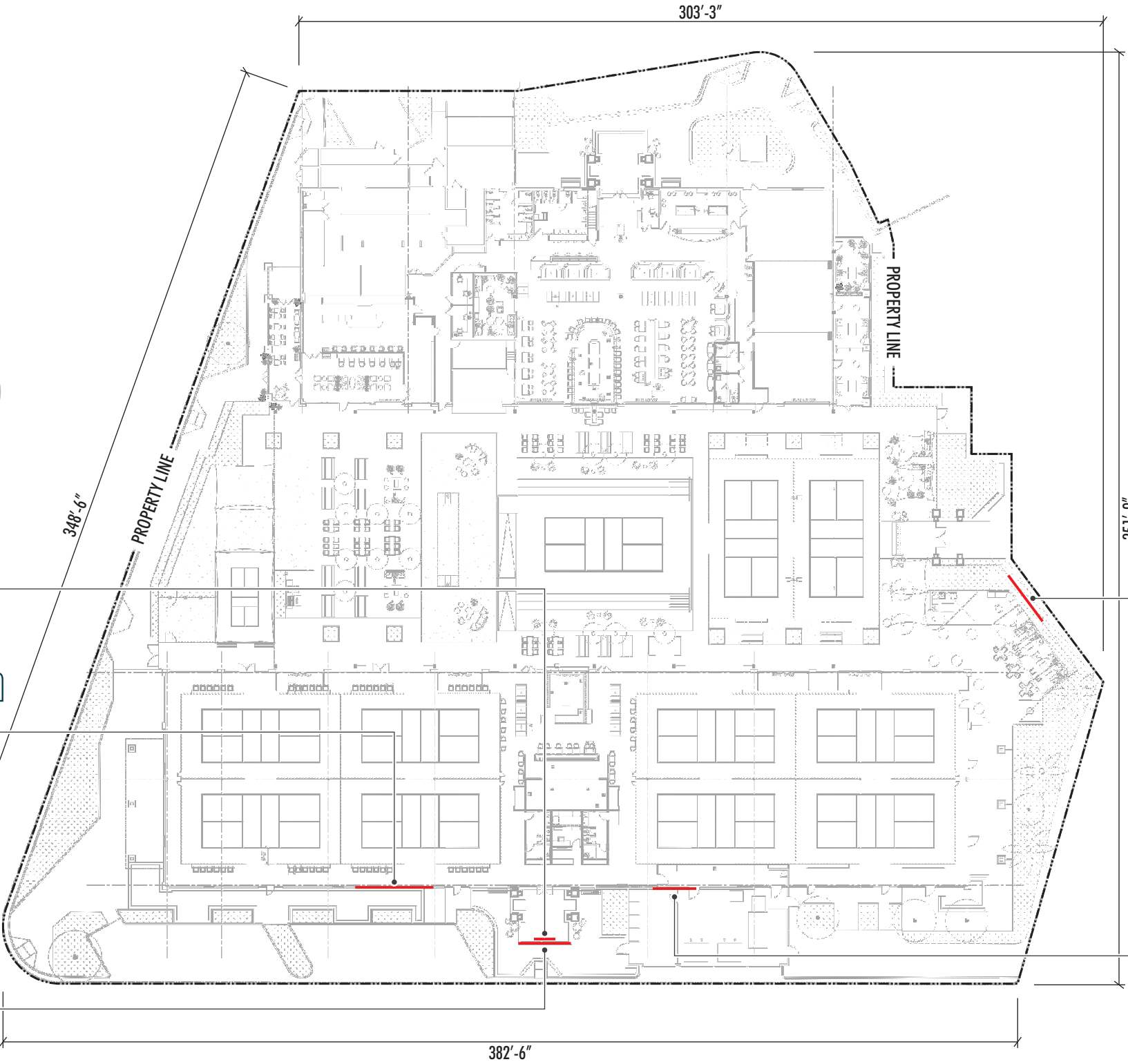
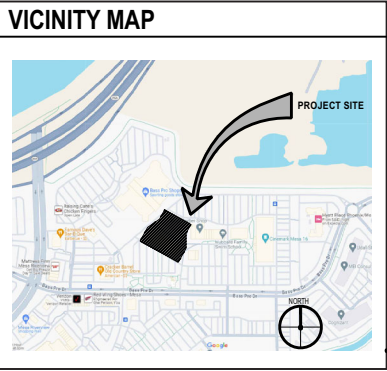
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☐ Conceptual Package

☒ Permit Package

☐ Production Package



# Indoor Pickle Pavillion



STL 101  
121.3 SQFT

IC 101  
66.4 SQFT

TL 101  
76.5 SQFT

ETL 101  
121.3 SQFT

BE 101  
49.3 SQFT



1 SITE PLAN  
SCALE: 1" = 50'-0"



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file path: \\ADS11\Design\ADS Drawings\Dink & Dine\Job 53303 (dwg 36984)\5 - permit

Client:  
**DADPP LLC**  
1213 Delaware St  
Huntington Beach, CA 92648

Project:  
**Dink and Dine**  
1017 N Dobson Rd & 1065 N Dobson Rd  
Mesa, AZ 85201

Sales Order(s): **36984**

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Drawing No: **36984-R06 (PS)**

Date: **02-11-25** BL&KB/RC/RT

Rev. Date: **04-30-25**

Sign Type: **Site Plan**

Sheet No: **SP1.00**

REQUIREMENTS FOR ELECTRICAL ILLUMINATION:

PRIMARY ELECTRICAL TO SIGN LOCATIONS TO BE PROVIDED BY OTHERS. A DEDICATED CIRCUIT WITH NO SHARED NEUTRALS AND A GROUND RETURNING TO THE PANEL IS REQUIRED FOR INSTALLATION.

NOTE: THIS PERTAINS TO THE SIGN'S INTERNAL WIRING ONLY, NOT THE PRIMARY WIRING. ALL TRANSFORMERS/DRIVERS/POWER SUPPLIES SHALL BE (GFI) GROUND FAULT-INTERRUPTED.

ALL SIGNS HAVE:

- DEDICATED BRANCH CIRCUIT
- THREE WIRES : LINE, NEUTRAL, & GROUND
- WIRE SIZE : MINIMUM OF 12 GA. THIN COPPER WIRE

ALSO NOTE: GAUGE OF WIRE IS DETERMINED BY THE LENGTH OF RUN & AMPERAGE AS PER NEC ARTICLE 300.

- GROUND WIRE MUST BE CONTINUOUS AND GO FROM THE SIGN TO THE PANELBOARD GROUND BUS.
- VOLTAGE SHOULD READ NO MORE THAN 3 VOLTS BETWEEN GROUND AND NEUTRAL.
- CONDUIT CAN NOT BE USED AS GROUND PATH.
- POWER TO SIGN MUST BE DONE BY A LICENSED ELECTRICIAN OR ELECTRICAL CONTRACTOR.

- This sign is intended to be installed in accordance with the requirements of Article 600 of the National Electrical Code and/or other applicable local codes. This includes proper grounding and bonding of the sign.

- The location of the disconnect switch after installation shall comply with Article 600.8(A)(1) of the National Electrical Code.

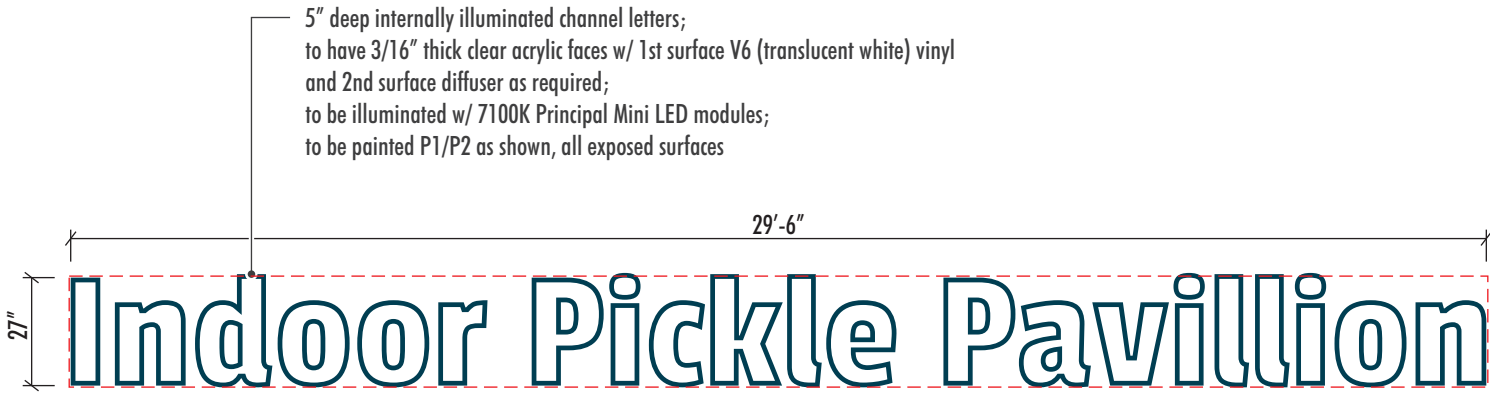
**1** Dedicated **20** Amp Circuit at **120** Volts

**E-Note #1:** All requirements not in sign company's scope of work must be in place prior to installation.

**E-Note #2:** Electrical to be brought to base of sign (by others) and not in sign company's scope of work.

**E-Note #3:** Timers and/or photocells for signs to be furnished and installed by the customer's electrician, who should also ensure that the completed electrical system is code compliant.

ALL COMPONENTS TO BE **UL** APPROVED  
ALL SIGNS TO BE **UL** LISTED

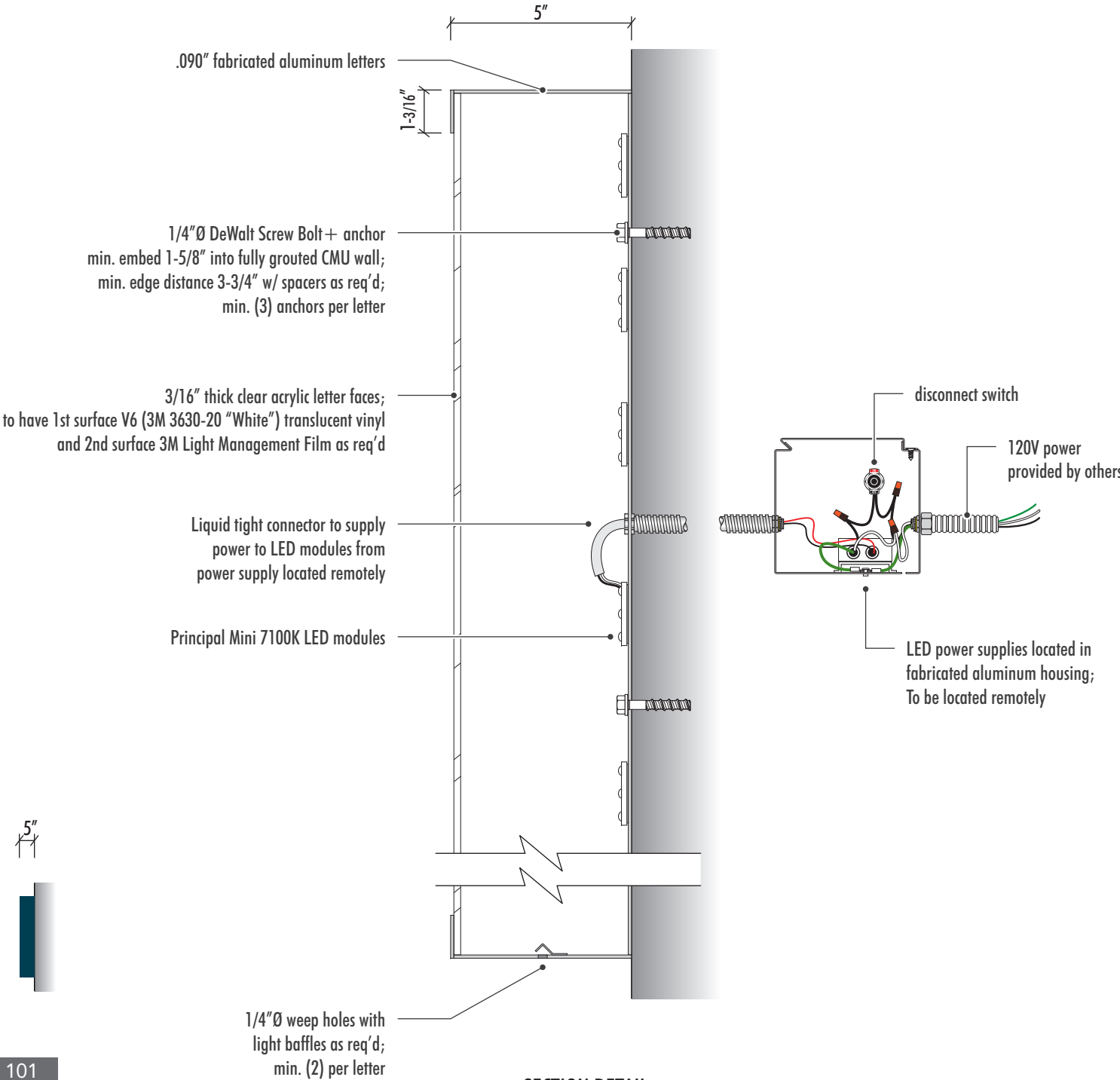


**1** SIGN IC – INDOOR PICKLE PAVILION  
SCALE: 1/4" = 1' - 0"

QTY: [01]  
66.4 SQFT

IC 101

APPLICATION # BOA25-00302



**2** SECTION DETAIL  
SCALE: 3" = 1' - 0"



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Drawing No: 36984-R06 (PS)

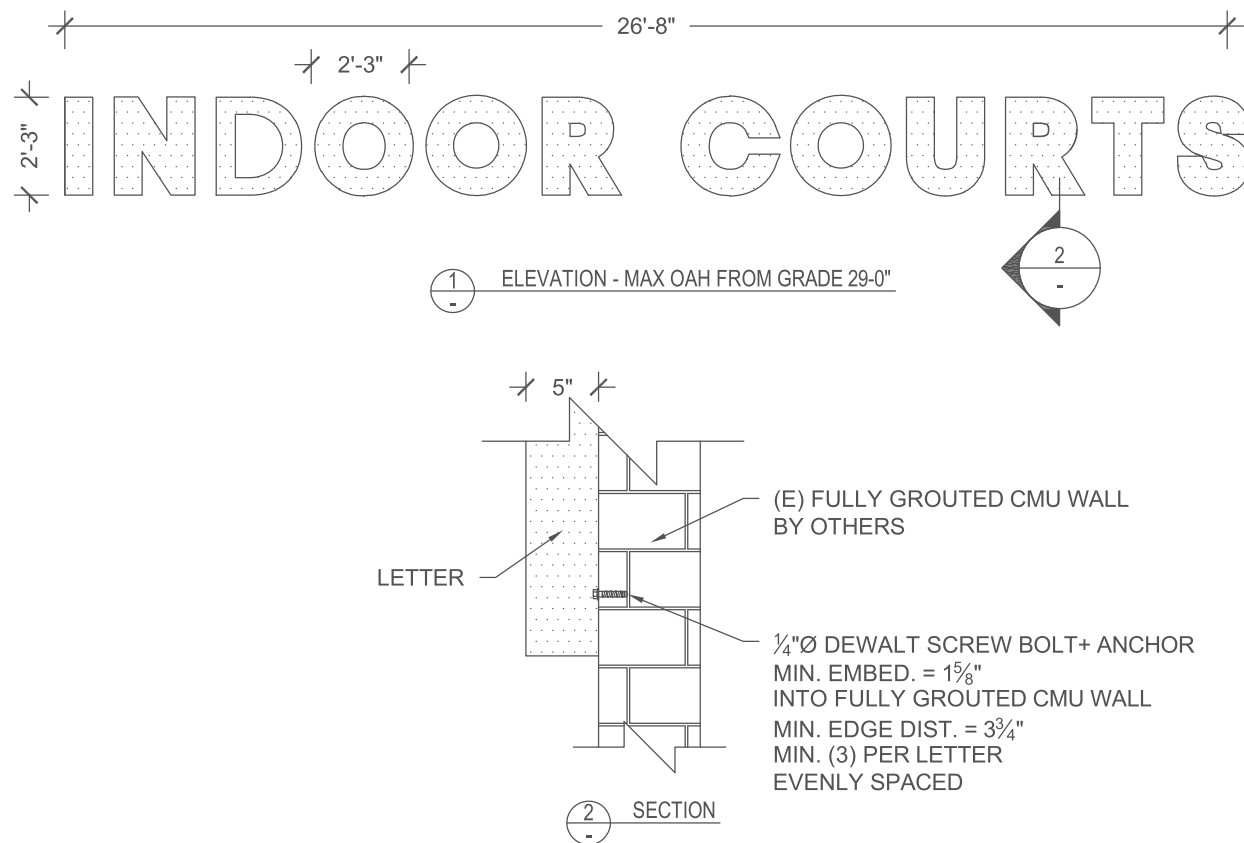
Date: 02-11-25 BL&KB/RC/RT

Rev. Date: 04-30-25

Sign Type: IC – Indoor Courts

Sheet No: IC.00 3 of 19

|            |   |                  |
|------------|---|------------------|
| PROJECT:   | DINK & DINE PICKLE PARK, IC - INDOOR COURTS - CHANNEL LETTER, 1065 N. DOBDSON RD., MESA, AZ | DATE: 12/05/2024 |
| PROJECT #: | 48951C  | ENGINEER: TH     |
| CLIENT:    | ARCHITECTURAL DESIGN & SIGN   | LAST REVISED:    |



## GENERAL NOTES

1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY 105 MPH EXPOSURE C
4. DEWALT SCREW-BOLT+ ANCHORS PER ICC-ESR-4042
5. CMU BLOCK SHALL CONFORM TO CHAPTER 21 OF IBC, ASTM C90, WITH STRENGTH F'M=1500 PSI MIN.
6. GROUT SHALL COMPLY PER IBC SECTION 2103.3
7. MORTAR SHALL COMPLY PER IBC SECTION 2103.2.1 AND MUST BE TYPE M, S OR N
8. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
9. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION
10. ALL EXISTING ELEMENTS AND DIMENSIONS TO BE VERIFIED IN FIELD



PROJECT: DINK & DINE PICKLE PARK  
PROJ. NO.: 48951C  
CLIENT: ARCHITECTURAL DESIGN & SIGN

DATE: 12/5/2024  
ENGINEER: TH

V5.8

units; pounds, feet unless noted otherwise

**Applied Wind Loads; from ASCE 7-16**

|                                      |        |  |
|--------------------------------------|--------|--|
| $p_{net} = \lambda K_{zt} p_{net30}$ |        | (ASCE 30.4-1)                            |
| $\lambda =$                          |        | (ASCE Fig. 30.4-1)                       |
| $K_{zt} =$                           | 1.0    | (unless unusual landscape)               |
| $V =$                                | 105    | Exposure = c                             |
| Area =                               | 1.4    | ft <sup>2</sup>                          |
| max. height =                        | 29.00  | ft                                       |
| $p_{net30} =$                        | 19.84  | psf                                      |
| $p_{net30} =$                        | -26.59 | psf                                      |
|                                      |        | $p_{net} = 26.79$ psf                    |
|                                      |        | <b><math>p_{net} = -35.90</math> psf</b> |

**Check 1/4" Dia. Dewalt Screw Bolt+ Anchor with Min. Embed.= 1.625" into CMU Wall: (ASD)**

|                                |                                  |                      |           |
|--------------------------------|----------------------------------|----------------------|-----------|
| Pnet=                          | See Above=                       | 35.90 psf            |           |
| Tributary Area for letter "O"= | $A_{Trib} = ((2'-3") * (2'-3"))$ | 5.06 ft <sup>2</sup> |           |
| Wind Load=                     | $WL = 0.6 * P_{net} * A_{Trib}$  | 109 lbs              |           |
| Dead Load=                     | $DL = 10 \text{psf} * A_{Trib}$  | 51 lbs               |           |
| #bolts=                        | =                                | 3 bolts              |           |
| Tension per bolt=              | $Ta = WL / \#bolts$              | 36 lbs               |           |
| Shear per bolt=                | $Va = DL / \#bolts$              | 17 lbs               |           |
| Tension capacity=              | (Per ICC ESR-4042) Tc=           | 315 lbs              |           |
| Shear capacity=                | Vc=                              | 400 lbs              |           |
| Combined Check:                | $Ta / Tc + Va / Vc$              | 0.16 < 1             | <b>OK</b> |



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Sales Order(s): 36984

Drawing No: 36984-R06 (PS)

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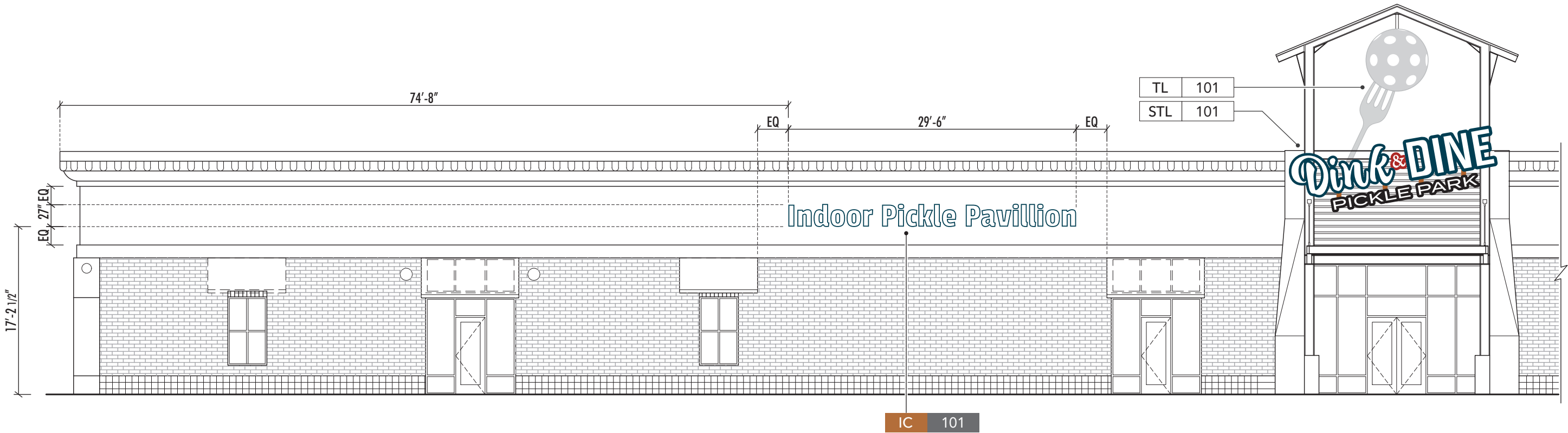
|            |                       |             |
|------------|-----------------------|-------------|
| Date:      | 02-11-25              | BLGKB/RC/RT |
| Rev. Date: | 04-30-25              |             |
| Sign Type: | Indoor Courts - Calcs |             |
| Sheet No:  | IC.10                 | 4 of 19     |





proposed channel letter location

**A** PHOTO RENDERING – CURRENT CONDITIONS  
SCALE: NTS



**CE** CONTEXT ELEVATION, PARTIAL SOUTH ELEVATION  
SCALE: 3/32" = 1' - 0"



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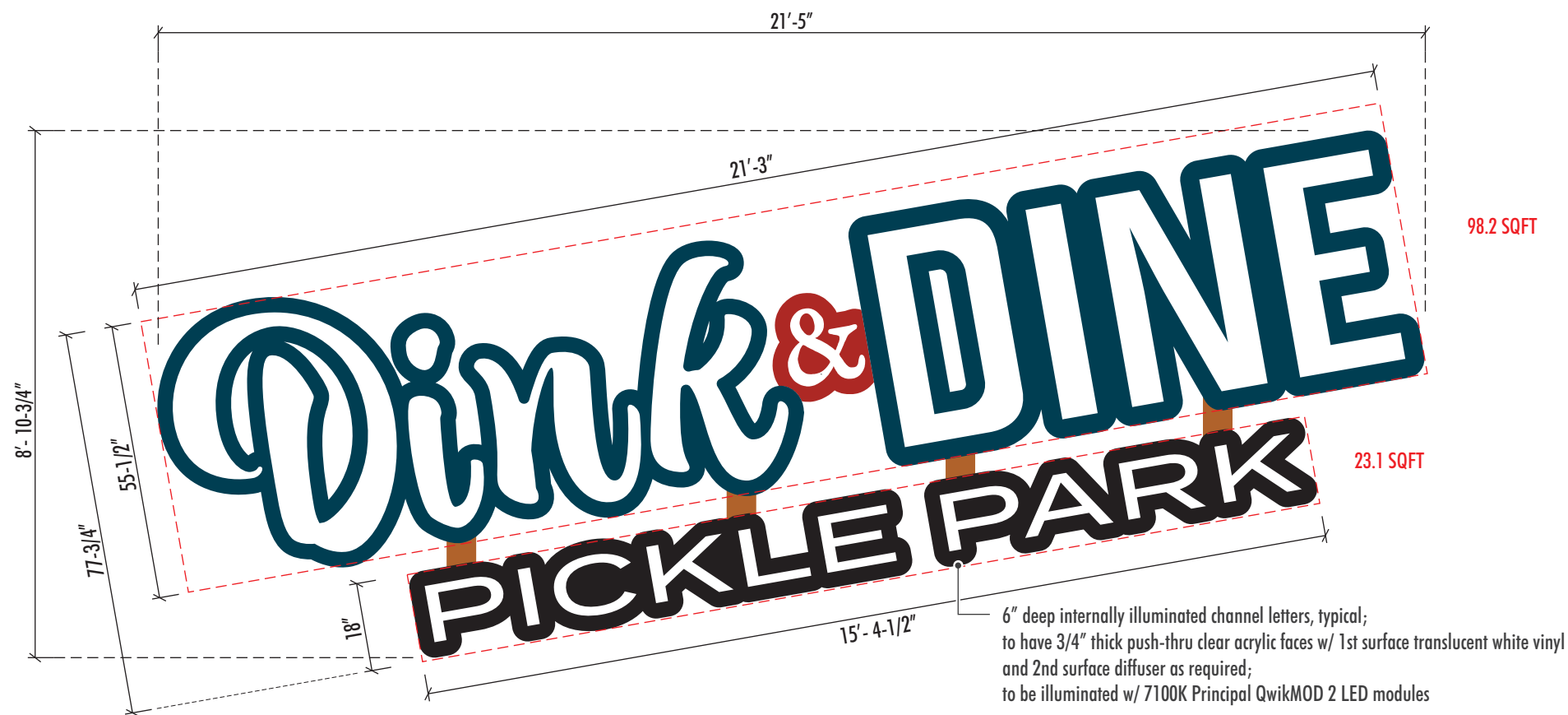
Drawing No: **36984-R06 (PS)**

Date: **02-11-25** BL&KB/RC/RT

Rev. Date: **04-30-25**

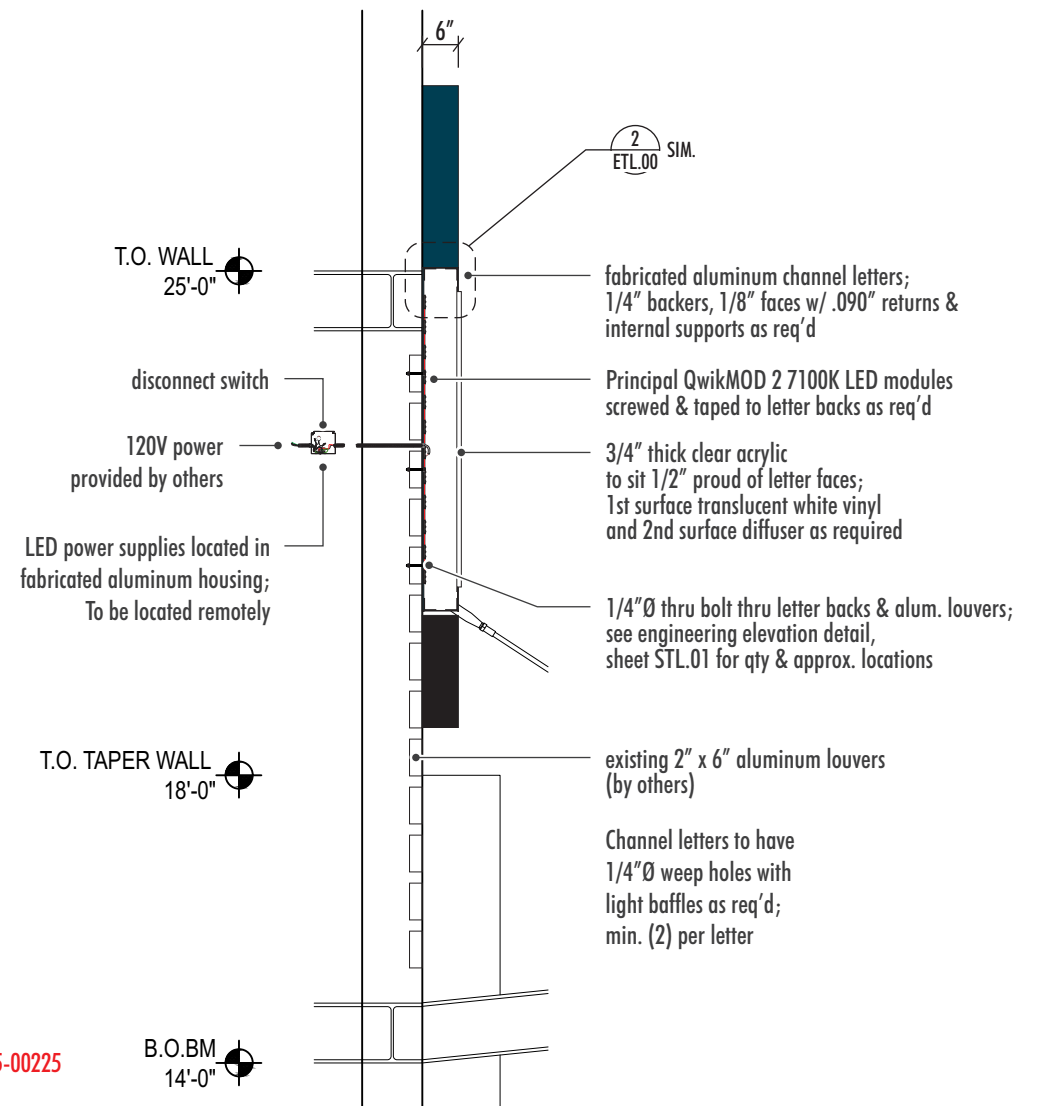
Sign Type: **Indoor Courts**

Sheet No: **IC.20**




1 SIGN STL – SOUTH TOWER LOGO (DINK & DINE PICKLE PARK)  
SCALE: 3/8" = 1' - 0"

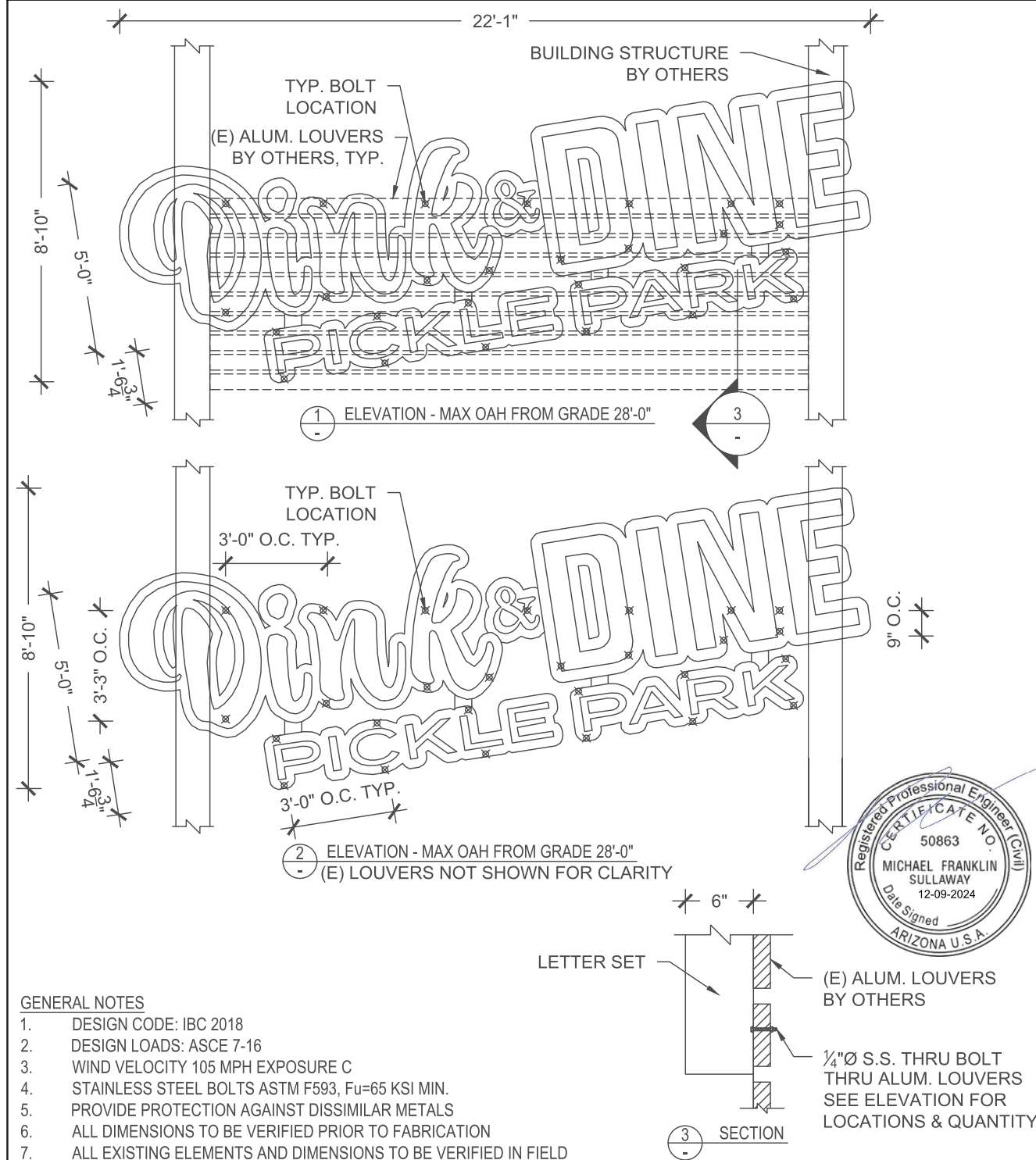
QTY: [01]    STL 101  
121.3 SQFT    APPLICATION # PRS25-00225



2 SIDE VIEW  
SCALE:  $\frac{3}{8}" = 1' - 0"$

|   |  |   |   |  |
|---|--|---|---|--|
|  <p>ARCHITECTURAL DESIGN &amp; SIGNS, INC.<br/>SIGNAGE + DIGITAL SOLUTIONS<br/>1.800.862.3202   <a href="http://WWW.AD-S.COM">WWW.AD-S.COM</a><br/>1160 RAILROAD STREET • CORONA, CA 92882<br/>5470 WYNN ROAD #600 • LAS VEGAS, NV 89118<br/>SAN FRANCISCO, CA &amp; SAN DIEGO, CA</p> <p>file path: \\ADS11\Design\ADS Drawings\Dink &amp; Dine\Job 53303 (dwg 36984)\5 - permit</p> | Client:  | Project:  | Sales Order(s): <b>36984</b><br><br>This is an original unpublished drawing created by AD/S Inc. It is submitted for your personal use in conjunction with a project being planned for you by AD/S Inc. It is not to be shown to anyone outside your organization, nor is it to be used, reproduced, copied or exhibited in any fashion without written permission from AD/S Inc. | Drawing No: <b>36984-R06 (PS)</b><br><br>Date: <b>02-11-25</b> BL&KB/RC/RT<br>Rev. Date: <b>04-30-25</b><br>Sign Type: <b>STL — South Tenant Logo</b><br>Sheet No: <b>STL.00</b> 6 of 19 |
|   | <b>DADPP LLC</b><br>1213 Delaware St<br>Huntington Beach, CA 92648 | <b>Dink and Dine</b><br>1017 N Dobson Rd & 1065 N Dobson Rd<br>Mesa, AZ 85201 |   |  |

|            |   |                  |
|------------|---|------------------|
| PROJECT:   | DINK & DINE PICKLE PARK, SIGN TYPE: STL- SOUTH TENANT LOGO, 1065 N. DOBDSON RD., MESA, AZ | DATE: 12/05/2024 |
| PROJECT #: | 48951B  | ENGINEER: HL     |
| CLIENT:    | ARCHITECTURAL DESIGN & SIGN   | LAST REVISED:    |



## GENERAL NOTES

1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY 105 MPH EXPOSURE C
4. STAINLESS STEEL BOLTS ASTM F593, Fu=65 KSI MIN.
5. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
6. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION
7. ALL EXISTING ELEMENTS AND DIMENSIONS TO BE VERIFIED IN FIELD

PROJECT: DINK & DINE PICKLE PARK  
PROJ. NO.: 48951B  
CLIENT: ARCHITECTURAL DESIGN & SIGN

DATE: 12/05/2024  
ENGINEER: HL

V5.8

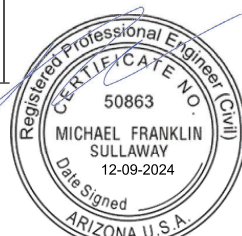
units; pounds, feet unless noted otherwise

**Applied Wind Loads; from ASCE 7-16**

|  |                      |                            |                   |
|--|----------------------|----------------------------|-------------------|
| p <sub>net</sub> =λ K <sub>zt</sub> p <sub>net30</sub> |                      | (ASCE 30.4-1)              |                   |
| λ=   | 1.35                 | (ASCE Fig. 30.4-1)         |                   |
| K <sub>zt</sub> =                                      | 1.0                  | (unless unusual landscape) |                   |
| V=   | 105 mph              | Exposure=                  | C                 |
| Area=  | 2.12 ft <sup>2</sup> |                            |                   |
| max. height=   | 28.00 ft             |                            |                   |
| p <sub>net30</sub> =                                   | 19.84 psf            | p <sub>net</sub> =         | 26.79 psf         |
| p <sub>net30</sub> =                                   | -26.59 psf           | p <sub>net</sub> =         | <b>-35.90 psf</b> |

**Check 0.25" Dia. S.S. Thru bolts F593 - Letter Set to Alum. Louvers (LRFD):**  $\phi = 0.75$

|   |  |                       |    |
|---|--|-----------------------|----|
| Pnet=                                       | See Above=                             | 35.90 psf             |    |
| Tributary Area Per Bolt (A <sub>1</sub> )=  | A <sub>Trib1</sub> =(3'-0")*(3'-3")=   | 9.750 ft <sup>2</sup> |    |
| Tributary Area Per Strip (A <sub>2</sub> )= | A <sub>Trib2</sub> =Area from AutoCAD= | 7.930 ft <sup>2</sup> |    |
| Wind Load (WL <sub>1</sub> )=               | WL1=Pnet*A <sub>Trib1</sub> =          | 0.350 kips            |    |
| Wind Load (WL <sub>2</sub> )=               | WL2=Pnet*A <sub>Trib2</sub> =          | 0.285 kips            |    |
| Dead Load (DL <sub>1</sub> )=               | DL1=1.2*10psf*A <sub>Trib1</sub> =     | 0.117 kips            |    |
| Dead Load (DL <sub>2</sub> )=               | DL2=1.2*10psf*A <sub>Trib2</sub> =     | 0.095 kips            |    |
| arm=  | from AutoCAD=                          | 23.75 in              |    |
| MWL=  | WL <sub>2</sub> *arm=                  | 6.762 k-in            |    |
| arm=  | (6")/2=                                | 3 in                  |    |
| MDL=  | DL <sub>2</sub> *arm=                  | 0.285 k-in            |    |
| Governing Spacing=                          | =                                      | 9 in                  |    |
| Additional tension due to WL=               | TWL=MWL/spacing/1 bolt=                | 0.751 kips            |    |
| Governing Spacing=                          | =                                      | 9 in                  |    |
| Additional tension due to DL=               | TDL=MDL/spacing/1 bolt=                | 0.032 kips            |    |
| #bolts=                                     | =                                      | 1 bolt                |    |
| Tu=   | WL <sub>1</sub> /#bolts+TWL+TDL=       | 1.133 kips            |    |
| Vu=   | DL <sub>1</sub> /#bolts=               | 0.117 kips            |    |
| dia.=                                       | =                                      | 0.250 in              |    |
| Abolt=                                      | =                                      | 0.049 in <sup>2</sup> |    |
| Fnt=  | (0.75*65ksi)=                          | 48.75 ksi             |    |
| Fnv=  | (0.45*65ksi)=                          | 29.25 ksi             |    |
| φTn=  | φ*Fnt*Abolt=                           | 1.795 kips            | ok |
| φVn=  | φ*Fnv*Abolt=                           | 1.077 kips            | ok |
| <u>Combined Check:</u>                      |  |                       |    |
| Fv=   | Vu/Abolt=                              | 2.384 ksi             |    |
| F'nt=                                       | 1.3Fnt-Fnt*Fv/φFnv≤Fnt=                | 49 ksi                |    |
| φTn=  | φ*F'nt*Abolt=                          | 1.795 kips            | ok |

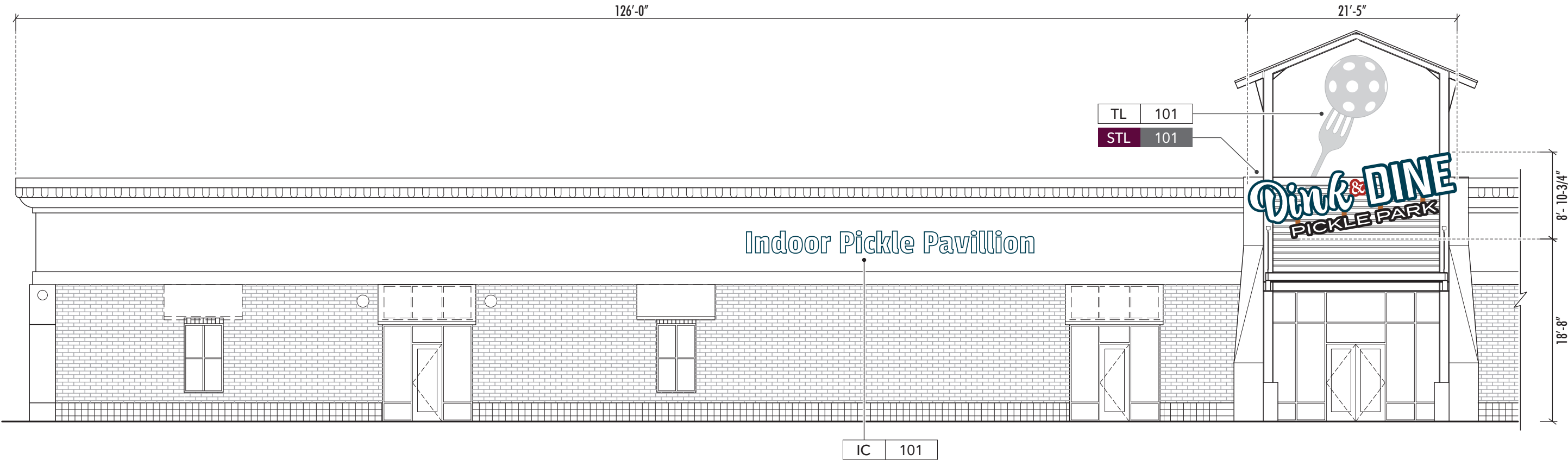






proposed logo location  
on new tower construction

A PHOTO RENDERING – CURRENT CONDITIONS  
SCALE: NTS



CE CONTEXT ELEVATION, PARTIAL SOUTH ELEVATION  
SCALE: 3/32" = 1' - 0"



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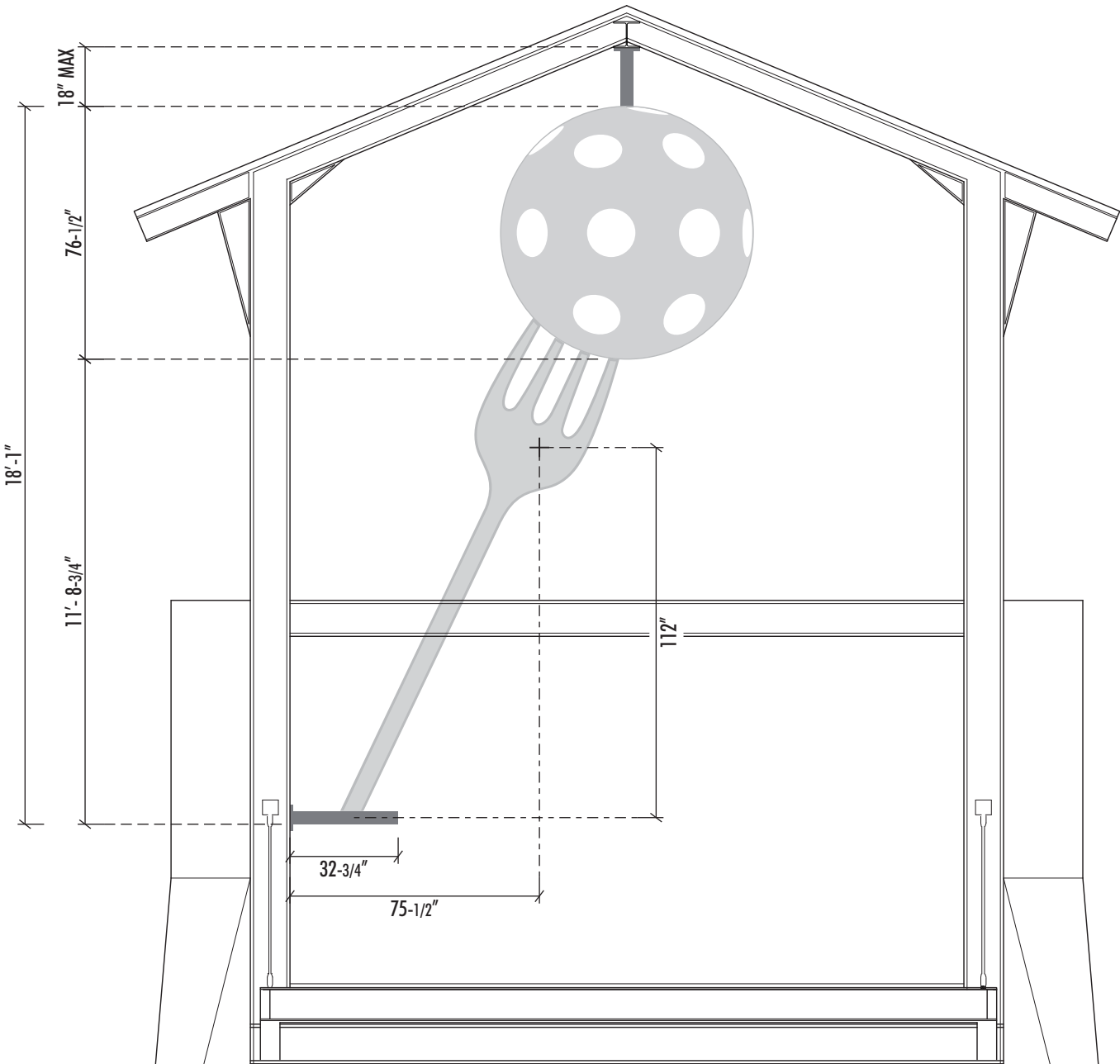
Date: 02-11-25 BL&KB/RC/RT

Rev. Date: 04-30-25

Sign Type: South Tenant Logo - Elevation

Sheet No: STL.20





1 SIGN TL – TOWER LOGO (PICKLE BALL AND FORK)  
SCALE: 1/4" = 1' - 0"

QTY: [01] TL 101  
76.5 SQFT APPLICATION # BOA25-00302

Section 11-41-7(A)(6)(C) – (76.5 + 76.5 + 76.5 + 76.5) / 2 = 153  
Section 11-43-9(B) – 153 / 2 = 76.5

MZO Section 11-41-7(A)(6)(C) – Spherical, free-form, sculptural, or other non-planar sign area is measured as fifty percent (50%) of the sum of the areas using only the four (4) vertical sides of the smallest four (4) vertical sided polyhedron that encompasses the sign structure (six (6) total sides when counting top and bottom)[See Figure 11-41-7-D]. No more than four vertical flat polyhedron faces are used to calculate the sign area for a non-planr sign.

MZO Section 11-43-9(B) – Fifty percent (50%) of the area of the three-dimensional figure shall be deducted from the total allowable sign area for the development.

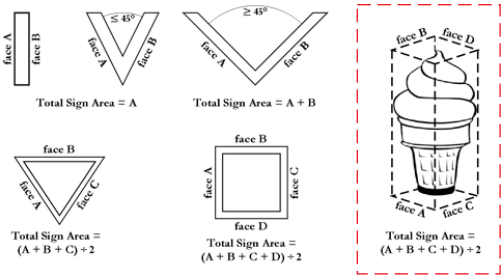
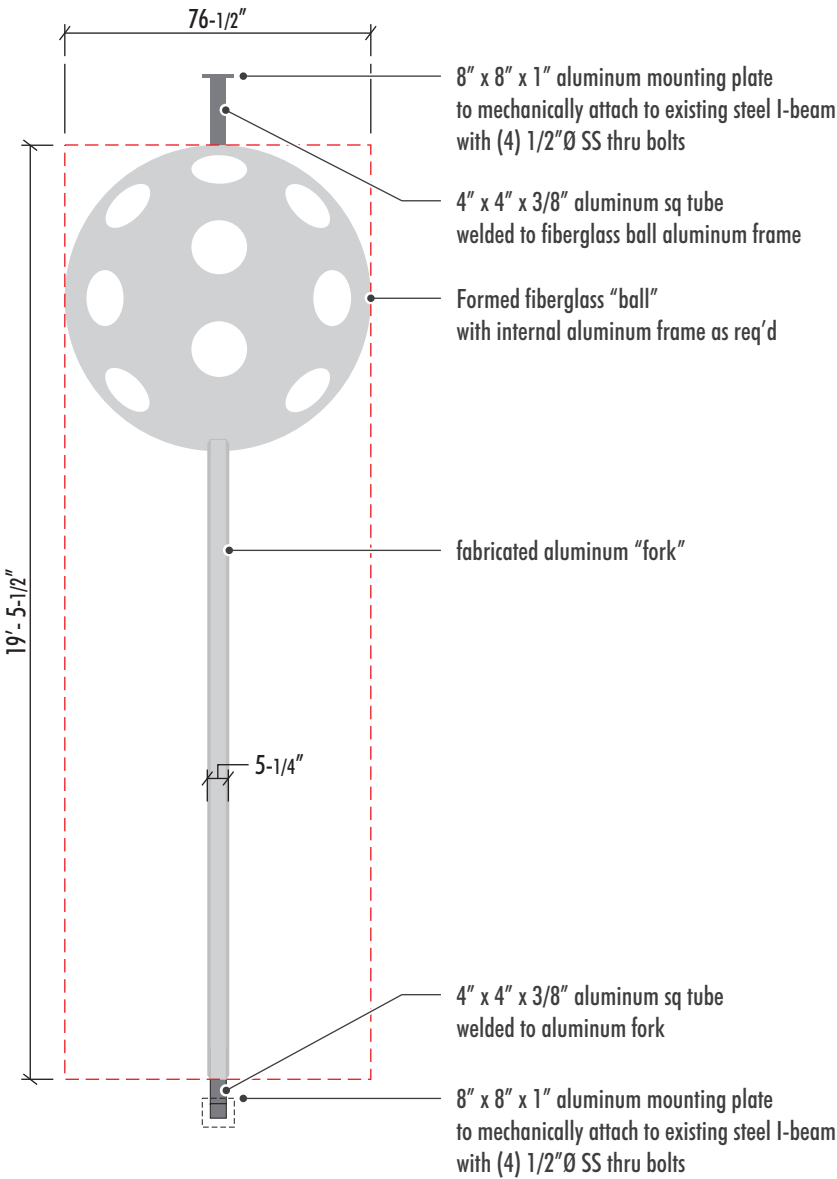
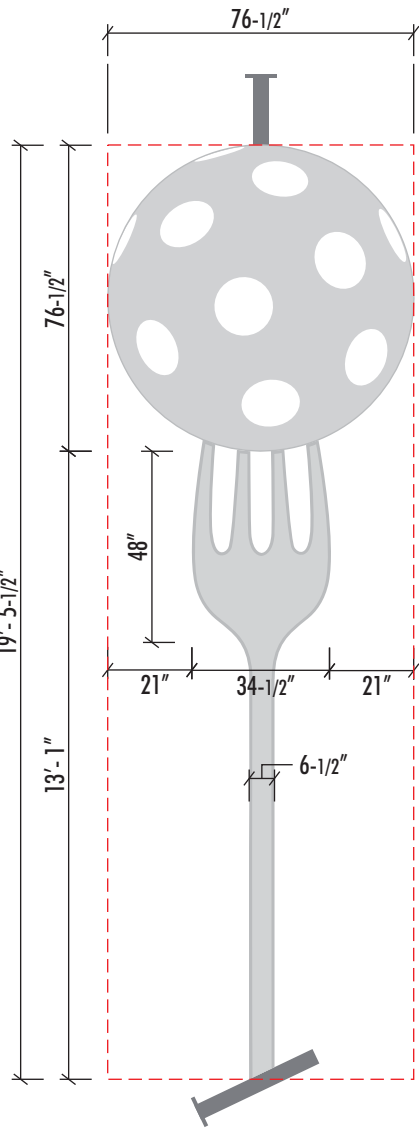


Figure 11-41-7-D



2 DETAIL – FACES A & D  
SCALE: 1/4" = 1' - 0"



3 DETAIL – FACES B & C  
SCALE: 1/4" = 1' - 0"



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file path: \\ADS11\Design\ADS Drawings\Dink & Dine\Job 53303 (dwg 36984)\5 - permit

Client:  
DADPP LLC  
1213 Delaware St  
Huntington Beach, CA 92648

Project:  
Dink and Dine  
1017 N Dobson Rd & 1065 N Dobson Rd  
Mesa, AZ 85201

Sales Order(s): 36984

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Drawing No: 36984-R06 (PS)

Date: 02-11-25

BL&KB/RC/RT

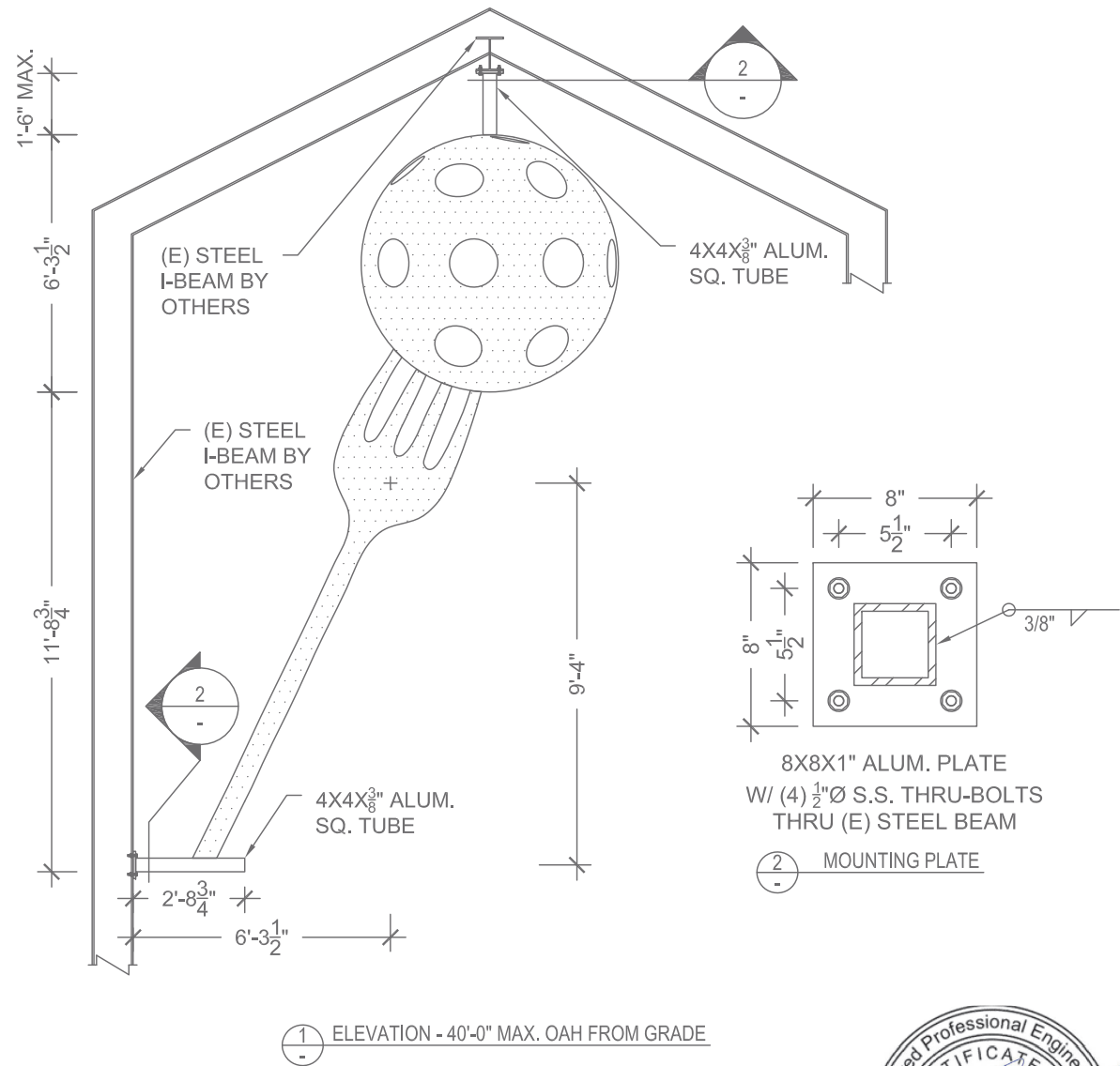
Rev. Date: 04-30-25

Sign Type: TL – Tower Logo

Sheet No: TL.00

PROJECT: DINK & DINE PICKLE PARK - 1065 N. DOBDSON RD., MESA, AZ  
PROJECT #: 48951E  
CLIENT: ARCHITECTURAL DESIGN & SIGN

DATE: 12/13/2024  
ENGINEER: TH  
LAST REVISED:



GENERAL NOTES

- DESIGN CODE: IBC 2018
- DESIGN LOADS: ASCE 7-16
- WIND VELOCITY: 105 MPH EXPOSURE C
- STAINLESS STEEL BOLTS ASTM F593, Fu=65 KSI MIN.
- ALUMINUM ELEMENTS 6061-T6
- PROVIDE PROTECTION AGAINST DISSIMILAR METALS USING ANTI-CORROSIVE PAINT OR NEOPRENE GASKETS.
- ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION.
- ALL EXISTING ELEMENTS AND DIMENSIONS TO BE VERIFIED IN FIELD.

PROJECT: DINK & DINE PICKLE PARK  
PROJ. NO.: 48951E  
CLIENT: ARCHITECTURAL DESIGN & SIGN

DATE: 12/16/2024  
ENGINEER: TH

V5.5

units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16

|                                      |                      |                            |            |
|--------------------------------------|----------------------|----------------------------|------------|
| $p_{net} = \lambda K_{zt} p_{net30}$ |                      | (ASCE 30.4-1)              |            |
| $\lambda =$                          | 1.45                 | (ASCE Fig. 30.4-1)         |            |
| $K_{zt} =$                           | 1.0                  | (unless unusual landscape) |            |
| $V =$                                | 105 mph              | Exposure =                 | c          |
| Area =                               | 44.5 ft <sup>2</sup> |                            |            |
| max. height =                        | 40.0                 |                            |            |
| $p_{net30} =$                        | 18.00 psf            | $p_{net} =$                | 26.10 psf  |
| $p_{net30} =$                        | -22.84 psf           | $p_{net} =$                | -33.12 psf |

Check 1/2" dia. S.S. Thru-Bolts - Pickle Ball to Steel Beam = 0.75 (LRFD)

|                |  |                       |                         |                       |
|----------------|--|-----------------------|-------------------------|-----------------------|
| $P_{net} =$    | (see above) =                                | 33.12 psf             | $F_u =$                 | 65 ksi                |
| Trib. Area =   | $(6.29'/2)^2 + (4''/12)(1.5') =$             | 31.57 ft <sup>2</sup> | $F_{nt} = 0.75 * F_u =$ | 48.75 ksi             |
| wind force =   | $1 \times 31.565 \times 33.12 \text{ psf} =$ | 1045 lbs              | $F_{nv} = 0.45 * F_u =$ | 29.25 ksi             |
| arm =          | $((1' \ 6'') + (6.29'/2)) * 12'' =$          | 55.74 in              | bolt dia. =             | 0.500 in              |
| MWL =          | wind force * arm =                           | 58272 lbs in          | A bolt =                | 0.196 in <sup>2</sup> |
| spacing =      | s =  | 5.5 in                |                         |                       |
| TWL =          | MWL/s/2 bolts =                              | 5297 lbs              |                         |                       |
| dead load =    | $1.2 \times 31.565 \times 10 \text{ psf} =$  | 379 lbs               |                         |                       |
| # anchors =    | n =  | 4                     |                         |                       |
| T per anchor = | dead load / n + TWL =                        | 5392 lbs              | 5.39 kips               |                       |
| T capacity =   | $F_{nt} * A_{bolt} =$                        | 7179 lbs              | 7.18 kips               | OK                    |
| V per anchor = | wind load / n =                              | 261 lbs               | 0.26 kips               |                       |
| V capacity =   | $F_{nv} * A_{bolt} =$                        | 4307 lbs              | 4.31 kips               | OK                    |

Combined Tension & Shear Check:

|                 |   |           |    |
|-----------------|---|-----------|----|
| $f_{rv} =$      | $V \text{ per bolt} / A(\text{bolt}) =$                   | 1.33 ksi  |    |
| $F'_{nt} =$     | $1.3F_{nt} - F_{nt} / \phi F_{nv} * f_{rv} \leq F_{nt} =$ | 49 ksi    |    |
| $\phi R_{nt} =$ | $\phi F'_{nt} * A(\text{bolt}) =$                         | 7.18 kips | OK |

Check 8x8x1" Aluminum Plate

|                      |       |   |                       |                |          |     |      |     |         |
|----------------------|-------|---|-----------------------|----------------|----------|-----|------|-----|---------|
| n =                  | 2.000 | arm =   | 0.850 in              | b =            | 8.000 in | t = | 1 in | T = | 5.392 k |
| M <sub>plate</sub> = |       | T per bolt * n * arm =                              | 9.167 k-in            | (T= see above) |          |     |      |     |         |
| S =                  |       | bt <sup>2</sup> /6 =                                | 1.333 in <sup>3</sup> |                |          |     |      |     |         |
| $\phi M_n =$         |       | $*F_y * S * 1.5 = 0.9 * 11 \text{ ksi} * S * 1.5 =$ | 19.800 k-in           | OK             |          |     |      |     |         |



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Client:  
**DADPP LLC**  
1213 Delaware St  
Huntington Beach, CA 92648

Project:  
**Dink and Dine**  
1017 N Dobson Rd & 1065 N Dobson Rd  
Mesa, AZ 85201

Sales Order(s): **36984**

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Drawing No: **36984-R06 (PS)**

Date: **02-11-25** BL&KB/RC/RT

Rev. Date: **04-30-25**

Sign Type: **Pickle Ball w/ Fork Calcs**

Sheet No: **TL.10** 10 of 19

**Check 1/2" dia. S.S. Thru-Bolts - Fork to Steel Beam** = 0.75 (LRFD)

|               |                                      |                       |            |              |                       |
|---------------|--------------------------------------|-----------------------|------------|--------------|-----------------------|
| Pnet=         | (see above)=                         | 33.12 psf             |            | Fu =         | 65 ksi                |
| Trib. Area=   | 15 ft <sup>2</sup> + (4")(2' 8.75")= | 15.91 ft <sup>2</sup> |            | Fnt=0.75*Fu= | 48.75 ksi             |
| wind force=   | 1 x 15.91 x 33.12 psf=               | 527 lbs               |            | Fnv=0.45*Fu= | 29.25 ksi             |
| arm=          | (9' 4")=                             | 112 in                |            | bolt dia.=   | 0.500 in              |
| MWL=          | wind force * arm=                    | 59017 lbs in          | 59.02 k in | A bolt=      | 0.196 in <sup>2</sup> |
| spacing=      | s=                                   | 5.5 in                |            |              |                       |
| TWL=          | MWL/s/2 bolts=                       | 5365 lbs              |            |              |                       |
| dead load=    | 1.2 x 15.91 x 10 psf=                | 191 lbs               |            |              |                       |
| arm=          | (6' 3.5")=                           | 75.5 in               |            |              |                       |
| MDL=          | dead load * arm=                     | 14414 lbs in          | 14.41 k in |              |                       |
| spacing=      | s=                                   | 5.5 in                |            |              |                       |
| TDL=          | MDL/s/2=                             | 1310 lbs              |            |              |                       |
| # anchors=    | n=                                   | 4                     |            |              |                       |
| T per anchor= | TWL+TDL=                             | 6676 lbs              | 6.68 kips  |              |                       |
| T capacity=   | Fnt*Abolt=                           | 7179 lbs              | 7.18 kips  | <b>OK</b>    |                       |
| V per anchor= | wind load/n+dead load/n=             | 179 lbs               | 0.18 kips  |              |                       |
| V capacity=   | Fnv*Abolt=                           | 4307 lbs              | 4.31 kips  | <b>OK</b>    |                       |

Combined Tension & Shear Check:

|        |                               |           |           |
|--------|-------------------------------|-----------|-----------|
| frv =  | V per bolt / A(bolt) =        | 0.91 ksi  |           |
| F'nt = | 1.3Fnt - Fnt/φFnv*frv ≤ Fnt = | 49 ksi    |           |
| φRnt = | φF'nt*A(bolt) =               | 7.18 kips | <b>OK</b> |

**Check 8x8x1" Aluminum Plate**

$n = 2.000$        $\text{arm} = 0.850 \text{ in}$        $b = 8.000 \text{ in}$        $t = 1 \text{ in}$        $T = 6.676 \text{ k}$   
 $M_{\text{plate}} = T \text{ per bolt} * n * \text{arm} = 11.349 \text{ k-in (T= see above)}$   
 $S = \frac{bt^3}{12} = 1.333 \text{ in}^3$   
 $\phi M_n = *F_y * S * 1.5 = 0.9 * 11 \text{ ksi} * S * 1.5 = 19.800 \text{ k-in} \quad \text{OK}$

**PROJECT:** DINK & DINE PICKLE PARK  
**PROJ. NO.:** 48951E  
**CLIENT:** ARCHITECTURAL DESIGN & SIGN

DATE: 12/13/2024  
ENGINEER: TH

Check 4"x4"x0.375" Sq. Aluminum Tube

(ADM 2015 - Ch F)

Top Outrigger: Mu=MWL= 58.27 k-in

| d | t     |
|---|-------|
| 4 | 0.375 |

Bottom Outrigger:  $M_u = M_{WL} + M_{DL} = 73.43 \text{ k-in} < \text{governs}$

welded? ☒ yes

$F_{cy} = F_{ty} =$  15 ksi

Mu = 73.43 k-in

Yielding: (Governs)  
(ADM 15, F.2)

|                          |            |
|--------------------------|------------|
| $M_n = Z F_{cy} =$       | 111.3 k-in |
| $M_n = 1.5 S_t F_{ty} =$ | 135.4 k-in |

Mn = 100.1 k-in OK

LB:  
(ADM 15 F.3.1)

|      |          |
|------|----------|
| Bp = | 19.9 ksi |
| Dp = | 0.11 ksi |
| Cp = | 122.7    |

$$\begin{aligned} 1 &= \text{Bp-Fcy}/1.6\text{Dp} = 28.4 \\ 2 &= k1 \text{ Bp}/1.6\text{Dp} = 57.4 \end{aligned}$$

since  $b/t < 1$ :  $F_c = F_{cy} = 15.00$

Bbr = 26.4 ksi  
Dbr = 0.17 ksi  
Cbr = 107

$$\begin{aligned} 1 &= \text{Bbr} \cdot 1.5 \text{ Fcy/mDbr} = 36.4 \\ 2 &= k_1 \text{ Bbr/mDbr} = 123.1 \end{aligned}$$

since  $b/t < 1$ :  $F_c = 1.5 F_{cy} = 22.50$

$$\begin{aligned} \text{Mnlb} &= F_c I_f / ccf + F_b I_w / ccw = 235.02 \text{ k-in} \\ \text{Mnlb} &= 211.51 \text{ k-in} \quad \text{OK} \end{aligned}$$


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**Client:**

**DADPP LLC**  
1213 Delaware St  
Huntington Beach, CA 92648

Project:

**Dink and Dine**  
1017 N Dobson Rd & 1065 N Dobson Rd  
Mesa, AZ 85201

Sales Order(s): 36984

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Drawing No: 36984-R06 (PS)

Date: 02-11-25 BL&KB/RC/RT

Rev. Date: 04-30-25

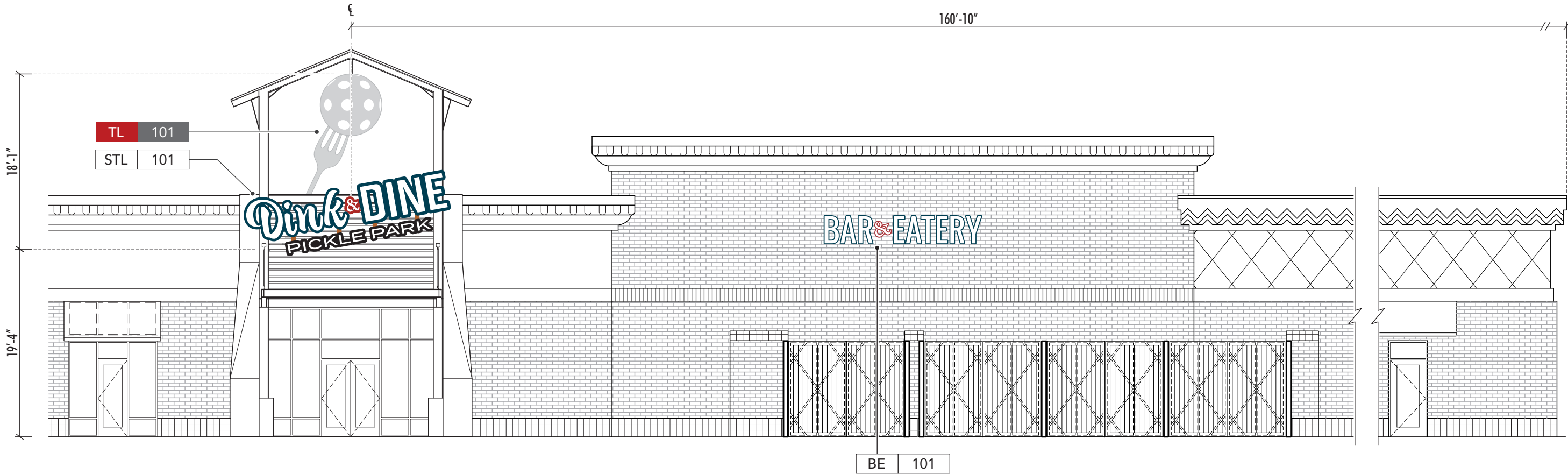
Sign Type: Pickle Ball w/ Fork Calcs

Sheet No: **TL.11** 11 of 19

proposed logo location  
on new tower construction



A PHOTO RENDERING  
SCALE: NTS



CE CONTEXT ELEVATION, PARTIAL SOUTH ELEVATION  
SCALE: 3/32" = 1' - 0"



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Client:

DADPP LLC  
1213 Delaware St  
Huntington Beach, CA 92648

Project:

Dink and Dine  
1017 N Dobson Rd & 1065 N Dobson Rd  
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Drawing No: 36984-R06 (PS)

Date: 02-11-25

BL&KB/RC/RT

Rev. Date: 04-30-25

Sign Type: South Tower Logo

Sheet No: TL.20

12 of 19



REQUIREMENTS FOR ELECTRICAL ILLUMINATION:

PRIMARY ELECTRICAL TO SIGN LOCATIONS TO BE PROVIDED BY OTHERS. A DEDICATED CIRCUIT WITH NO SHARED NEUTRALS AND A GROUND RETURNING TO THE PANEL IS REQUIRED FOR INSTALLATION.

**NOTE:** THIS PERTAINS TO THE SIGN'S INTERNAL WIRING ONLY, NOT THE PRIMARY WIRING. ALL TRANSFORMERS/DRIVERS/POWER SUPPLIES SHALL BE (GFI) GROUND FAULT-INTERRUPTED.

ALL SIGNS HAVE:

- DEDICATED BRANCH CIRCUIT
- THREE WIRES : LINE, NEUTRAL, & GROUND
- WIRE SIZE : MINIMUM OF 12 GA. THIN COPPER WIRE

ALSO NOTE: GAUGE OF WIRE IS DETERMINED BY THE LENGTH OF RUN & AMPERAGE AS PER NEC ARTICLE 300.

- GROUND WIRE MUST BE CONTINUOUS AND GO FROM THE SIGN TO THE PANELBOARD GROUND BUS.
- VOLTAGE SHOULD READ NO MORE THAN 3 VOLTS BETWEEN GROUND AND NEUTRAL.
- CONDUIT CAN NOT BE USED AS GROUND PATH.
- POWER TO SIGN MUST BE DONE BY A LICENSED ELECTRICIAN OR ELECTRICAL CONTRACTOR.

**E-Note #1:** All requirements **not** in sign company's scope of work must be in place prior to installation.

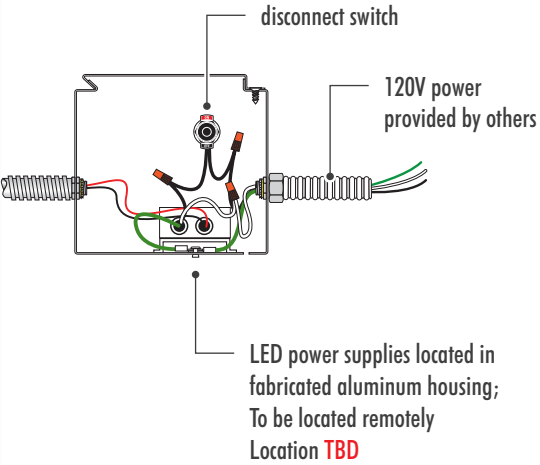
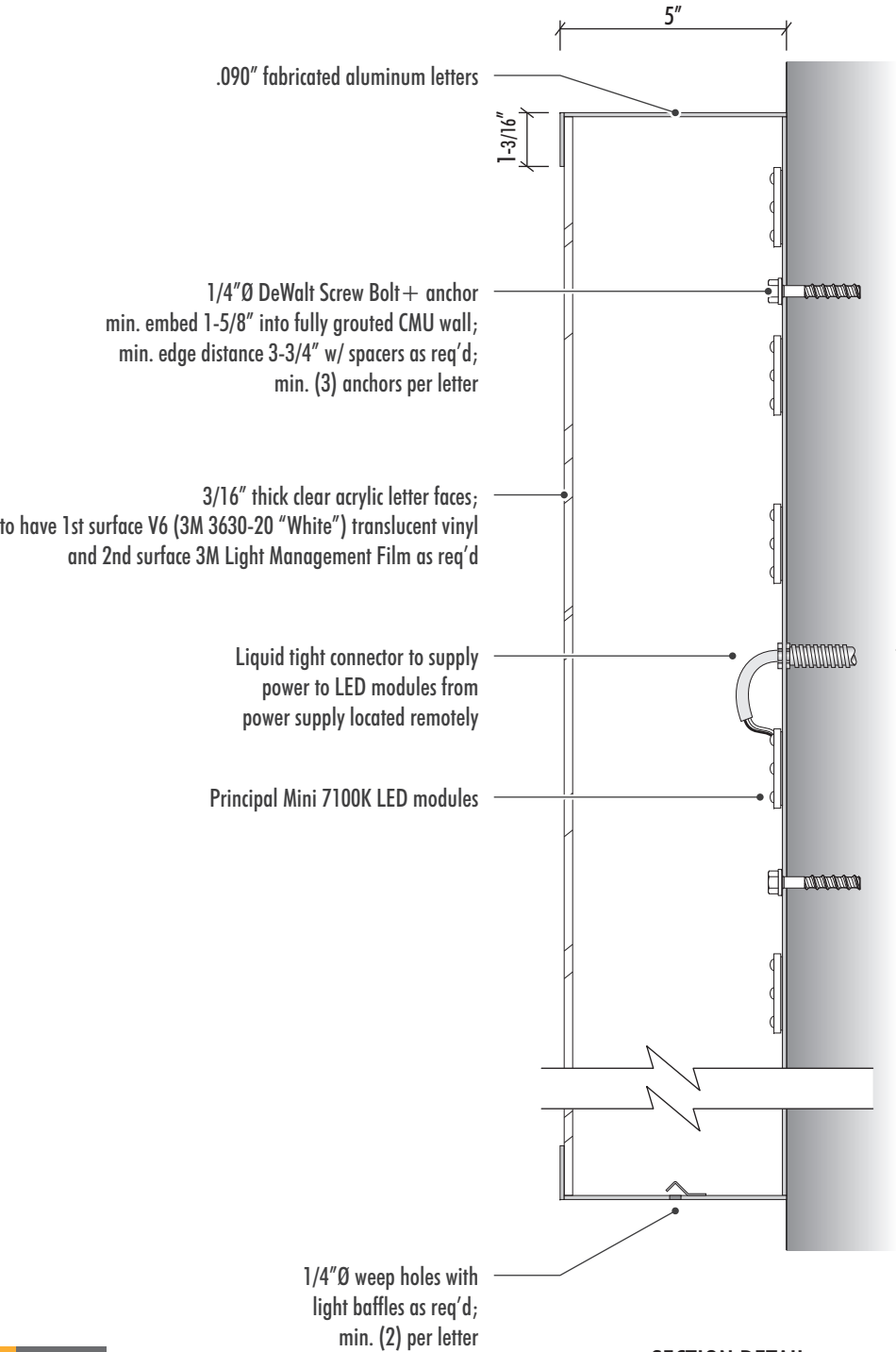
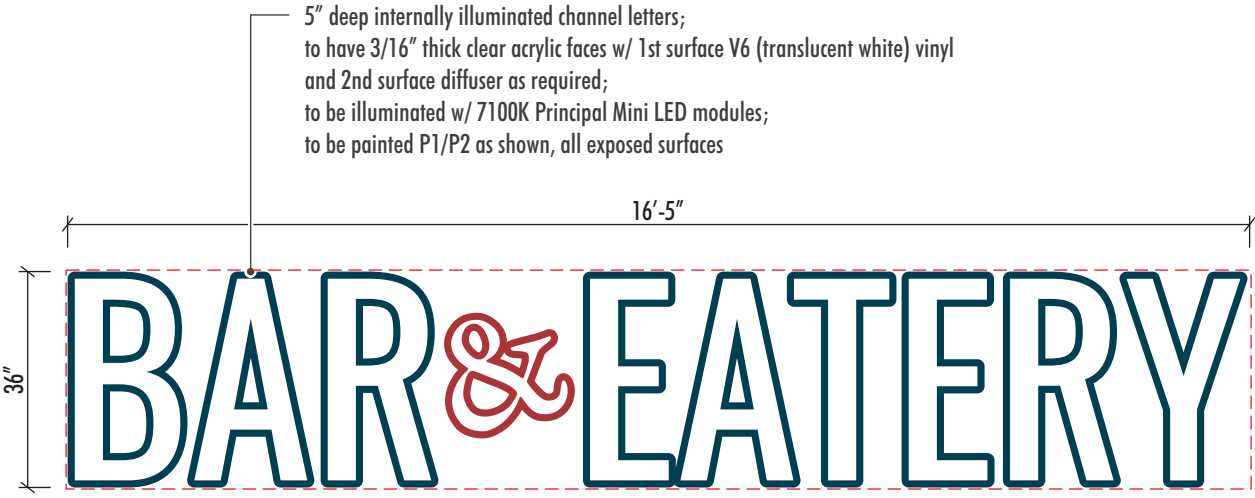
**E-Note #2:** Electrical to be brought to base of sign (by others) and **not** in sign company's scope of work.

**E-Note #3:** Timers and/or photocells for signs to be furnished and installed by the customer's electrician, who should also ensure that the completed electrical system is code compliant.

ALL COMPONENTS TO BE **UL** APPROVED

ALL SIGNS TO BE **UL** LISTED

**1** Dedicated **20** Amp Circuit at **120** Volts



**1** SIGN BE – BAR & EATERY ENTRY CHANNEL LETTERS  
SCALE: 3/8" = 1' - 0"

QTY: [01]  
49.3 SQFT

**BE** 101  
APPLICATION # BOA25-00302

**2** SECTION DETAIL  
SCALE: 3" = 1' - 0"



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Client:

**DADPP LLC**  
1213 Delaware St  
Huntington Beach, CA 92648

Project:

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1017 N Dobson Rd & 1065 N Dobson Rd  
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Drawing No: **36984-R06 (PS)**

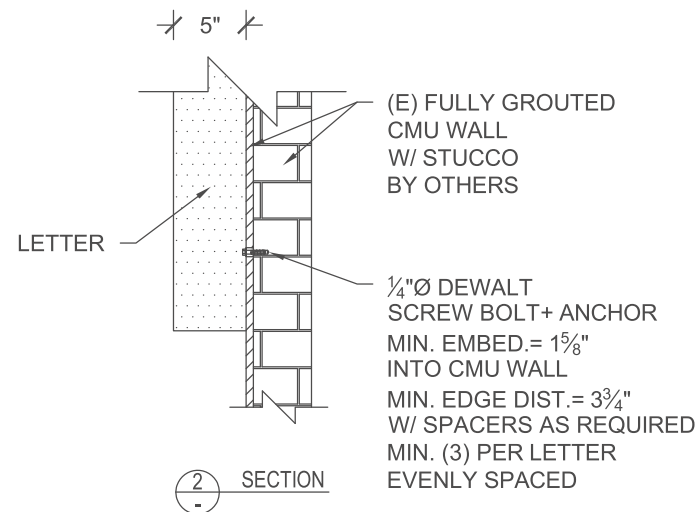
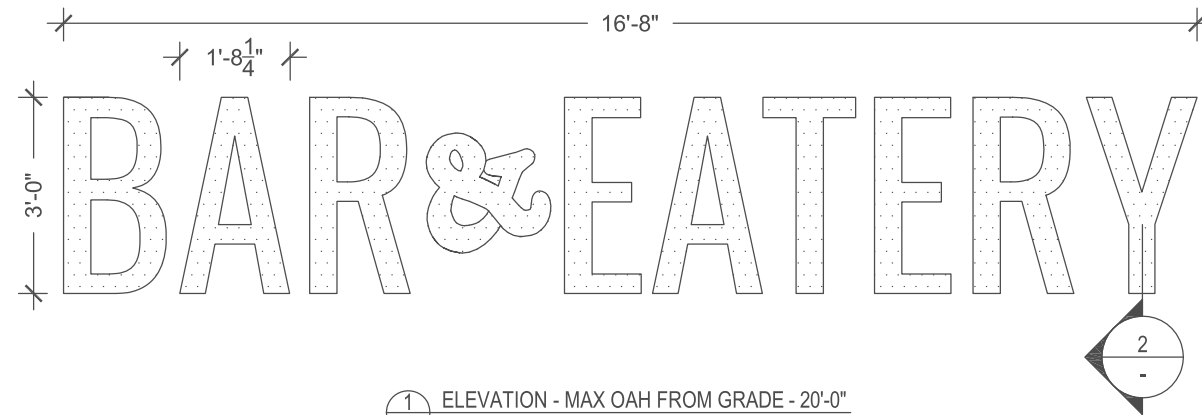
Date: **02-11-25** BL&KB/RC/RT

Rev. Date: **04-30-25**

Sign Type: **Bar & Eatery Entry**

Sheet No: **BE.00** 13 of 19

|            |   |               |            |
|------------|---|---------------|------------|
| PROJECT:   | DINK & DINE PICKLE PARK, SIGN BE - BAR & EATERY, 1065 N. DOBSON RD., MESA, AZ | DATE:         | 12/05/2024 |
| PROJECT #: | 48951F  | ENGINEER:     | JC         |
| CLIENT:    | ARCHITECTURAL DESIGN & SIGN   | LAST REVISED: |            |



## GENERAL NOTES

- (2) SECTION

-

  1. DESIGN CODE: IBC 2018
  2. DESIGN LOADS: ASCE 7-16
  3. WIND VELOCITY 105 MPH EXPOSURE C
  4. DEWALT SCREW-BOLT+ ANCHORS PER ICC-ESR-4042
  5. CMU BLOCK SHALL CONFORM TO CHAPTER 21 OF IBC, ASTM C90, WITH STRENGTH F'M=1500 PSI MIN.
  6. GROUT SHALL COMPLY PER IBC SECTION 2103.3
  7. MORTAR SHALL COMPLY PER IBC SECTION 2103.2.1 AND MUST BE TYPE M, S OR N
  8. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
  9. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION
  10. ALL EXISTING ELEMENTS AND DIMENSIONS TO BE VERIFIED IN FIELD



PROJECT: DINK & DINE PICKLE PARK  
 PROJ. NO.: 48951F  
 CLIENT: ARCHITECTURAL DESIGN & SIGN

V5.8

units; pounds, feet unless noted otherwise

### Applied Wind Loads; from ASCE 7-16

|                                      |                            |                                      |            |
|--------------------------------------|----------------------------|--------------------------------------|------------|
| $p_{net} = \lambda K_{zt} p_{net30}$ | (ASCE 30.4-1)              |                                      |            |
| $\lambda =$                          | (ASCE Fig. 30.4-1)         |                                      |            |
| $K_{zt} = 1.0$                       | (unless unusual landscape) |                                      |            |
| $V = 105$                            | Exposure =                 | c                                    |            |
| Area = 2.25                          | ft <sup>2</sup>            |                                      |            |
| max. height = 20.00                  | ft                         |                                      |            |
| $p_{net30} = 19.84$                  | psf                        | $p_{net} = 25.59$                    | psf        |
| $p_{net30} = -26.59$                 | psf                        | <b><math>p_{net} = -34.31</math></b> | <b>psf</b> |

**Check 1/4" Dia. Dewalt Screw Bolt+ into CMU - Min. Embed.=1.625" (ASD):**

|                   |                                |                       |           |
|-------------------|--------------------------------|-----------------------|-----------|
| Pnet=             | See Above=                     | 34.31 psf             |           |
| Tributary Area=   | $A_{Trib}=(3'-0")*(1'-8.25")=$ | 5.063 ft <sup>2</sup> |           |
| Wind Load=        | $WL=0.6*Pnet*A_{Trib}=$        | 104 lbs               |           |
| Dead Load=        | $DL=10psf*A_{Trib}=$           | 51 lbs                |           |
| #bolts=           | =                              | 3 bolts               |           |
| Tension per bolt= | $Ta=WL/\#bolts=$               | 35 lbs                |           |
| Shear per bolt=   | $Va=DL/\#bolts=$               | 17 lbs                |           |
| Tension capacity= | (Per ICC ESR-4042) Tc=         | 315 lbs               |           |
| Shear capacity=   | Vc=                            | 400 lbs               |           |
| Combined Check:   | $Ta/Tc+Va/Vc=$                 | 0.15 <1               | <b>ok</b> |



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Client:

**DADPP LLC**  
1213 Delaware St  
Huntington Beach, CA 92648

Project:

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**Dink and Dine**  
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Mesa, AZ 85201

Sales Order(s): 36984

Drawing No: 36984-R06 (PS)

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Date: 02-11-25 BL&KB/RC/RT

Rev. Date: 04-30-25

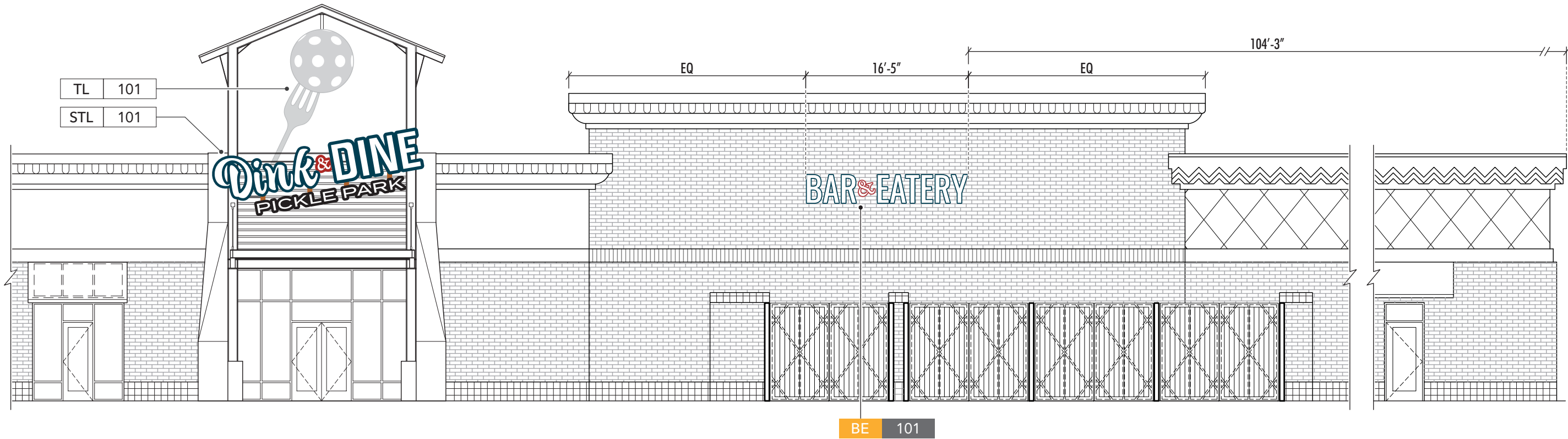
Sign Type: **Bar & Eatery Entry - Calcs**

Sheet No: **BE.10** 14 of 19



proposed channel letter location

A PHOTO RENDERING – CURRENT CONDITIONS  
SCALE: NTS



CE CONTEXT ELEVATION, PARTIAL SOUTH ELEVATION  
SCALE: 3/32" = 1' - 0"



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file path: \\ADS11\Design\ADS Drawings\Dink & Dine\Job 53303 (dwg 36984)\5 - permit

Client:

DADPP LLC  
1213 Delaware St  
Huntington Beach, CA 92648

Project:

Dink and Dine  
1017 N Dobson Rd & 1065 N Dobson Rd  
Mesa, AZ 85201

Sales Order(s): 36984

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Drawing No: 36984-R06 (PS)

Date: 02-11-25

BL&KB/RC/RT

Rev. Date: 04-30-25

Sign Type: Bar & Eatery Entry

Sheet No: BE.20

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REQUIREMENTS FOR ELECTRICAL ILLUMINATION:

PRIMARY ELECTRICAL TO SIGN LOCATIONS TO BE PROVIDED BY OTHERS. A DEDICATED CIRCUIT WITH NO SHARED NEUTRALS AND A GROUND RETURNING TO THE PANEL IS REQUIRED FOR INSTALLATION.

NOTE: THIS PERTAINS TO THE SIGN'S INTERNAL WIRING ONLY, NOT THE PRIMARY WIRING. ALL TRANSFORMERS/DRIVERS/POWER SUPPLIES SHALL BE (GFI) GROUND FAULT-INTERRUPTED. ALL SIGNS HAVE: - DEDICATED BRANCH CIRCUIT - THREE WIRES : LINE, NEUTRAL, & GROUND - WIRE SIZE : MINIMUM OF 12 GA. THIN COPPER WIRE

ALSO NOTE: GAUGE OF WIRE IS DETERMINED BY THE LENGTH OF RUN & AMPERAGE AS PER NEC ARTICLE 300. - GROUND WIRE MUST BE CONTINUOUS AND GO FROM THE SIGN TO THE PANELBOARD GROUND BUS. - VOLTAGE SHOULD READ NO MORE THAN 3 VOLTS BETWEEN GROUND AND NEUTRAL. - CONDUIT CAN NOT BE USED AS GROUND PATH. - POWER TO SIGN MUST BE DONE BY A LICENSED ELECTRICIAN OR ELECTRICAL CONTRACTOR.

- This sign is intended to be installed in accordance with the requirements of Article 600 of the National Electrical Code and/or other applicable local codes. This includes proper grounding and bonding of the sign. - The location of the disconnected switch after installation shall comply with Article 600.8(A)(1) of the National Electrical Code.

E-Note #1: All requirements **not** in sign company's scope of work must be in place prior to installation.

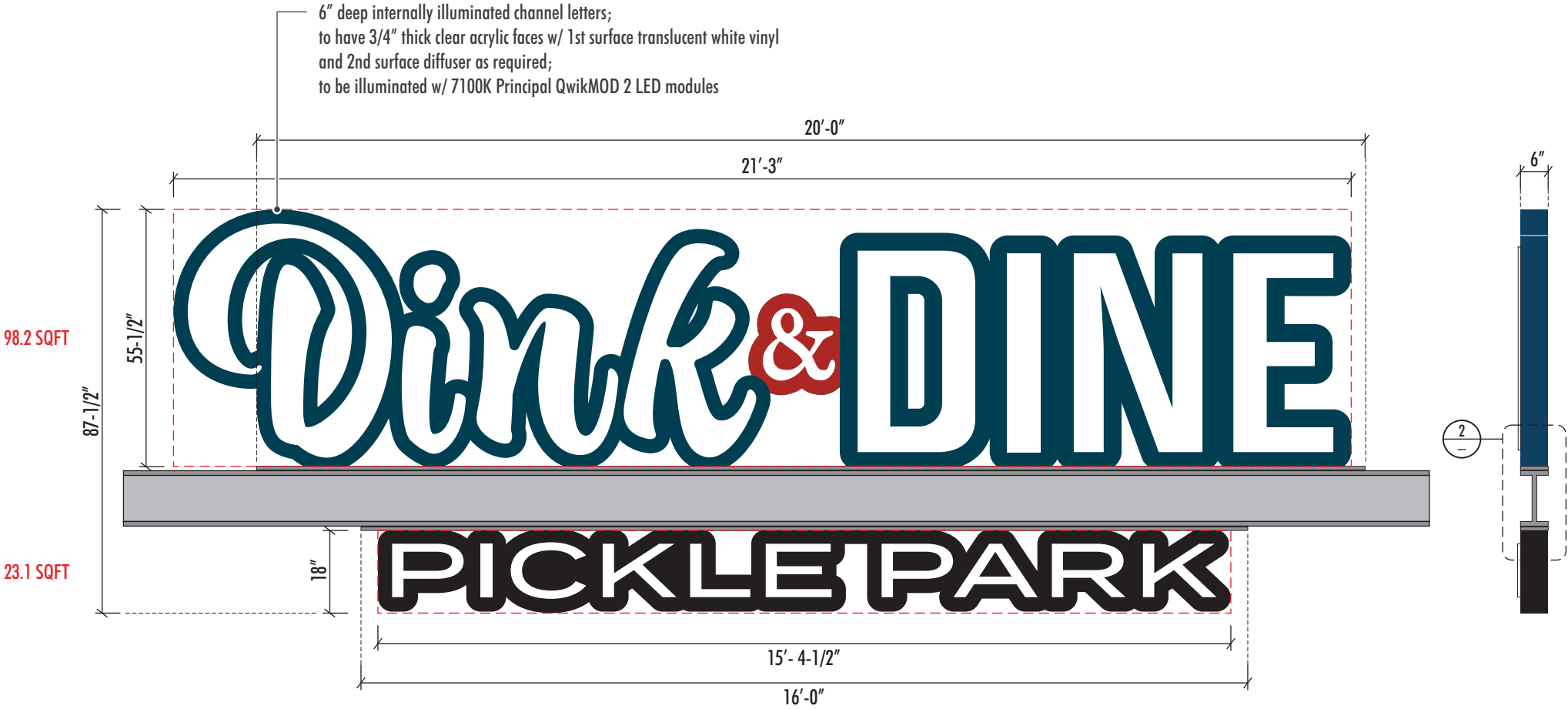
E-Note #2: Electrical to be brought to base of sign (by others) and **not** in sign company's scope of work.

E-Note #3: Timers and/or photocells for signs to be furnished and installed by the customer's electrician, who should also ensure that the completed electrical system is code compliant.

ALL COMPONENTS TO BE **UL** APPROVED

ALL SIGNS TO BE **UL** LISTED

**1** Dedicated **20** Amp Circuit at **120** Volts



fabricated aluminum channel letters; 1/4" backers, 1/8" faces w/ .090" returns & internal supports as req'd

Principal QwikMOD 2 7100K LED modules screwed & taped to letter backs as req'd

3/4" thick clear acrylic to sit 1/2" proud of letter faces; 1st surface translucent white vinyl and 2nd surface diffuser as required

low voltage power thru 1/2"Ø conduit from remote located transformer housing

3/8"Ø thru bolt thru alum. mount plate & existing steel beam; min. (1) front & back, spaced 2'-0" O.C.

20'-6" x 6" x 3/4" aluminum mounting plate

existing steel beam (by others)

low voltage power thru 1/2"Ø conduit from remote located transformer housing

16'-6" x 6" x 3/4" aluminum mounting plate

3/8"Ø thru bolt thru alum. mount plate & existing steel beam; min. (1) front & back, spaced 4'-0" O.C.

Channel letters to have 1/4"Ø weep holes with light baffles as req'd; min. (2) per letter

**1** SIGN ETL — DINK & DINE EAST TENANT LOGO  
SCALE: 3/8" = 1' - 0"

QTY: [01]  
121.3 SQFT

ETL 101  
APPLICATION # BOA25-00302

**2** SIDE VIEW  
SCALE: 3/8" = 1' - 0"



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Drawing No: 36984-R06 (PS)

Date: 02-11-25 BL&KB/RC/RT

Rev. Date: 04-30-25

Sign Type: ETL — East Tenant Logo

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SULLAWAY

ENGINEERING

Page 3 of 3

PROJECT: DINK & DINE PICKLE PARK

DATE: 12/05/2024

PROJ. NO.: 48951A

ENGINEER: JD

CLIENT: ARCHITECTURAL DESIGN & SIGN

v5.8

units; pounds, feet unless noted otherwise

Check 0.375" Dia. S.S. Thru bolts F593 - Alum. Plate to (E) I Beam (LRFD):

φ= 0.75

For Bottom letters:

Pnet=(See Page#2)=34.31 psf

Tributary Area= $A_{Trib}=(4'-0")*(1'-6.25")=6.083 \text{ ft}^2$

Wind Load= $WL=Pnet*ATrib=0.209 \text{ kips}$

Dead Load= $DL=1.2*10psf*ATrib=0.073 \text{ kips}$

arm= $(1'-6.25")/2=9.125 \text{ in}$

MWL= $WL*arm=1.904 \text{ k-in}$

Spacing= $=4 \text{ in}$

Additional tension due to WL= $TWL=MWL/spacing/1 \text{ bolt}=0.476 \text{ kips}$

#bolts= $=2 \text{ bolts}$

Tu= $DL/\#bolts+TWL=0.513 \text{ kips}$

Vu= $WL/\#bolts=0.104 \text{ kips}$

dia.= $=0.375 \text{ in}$

Abolt= $=0.110 \text{ in}^2$

Fnt= $(0.75*65ksi)=48.75 \text{ ksi}$

Fnv= $(0.45*65ksi)=29.25 \text{ ksi}$

φTn= $φ*Fnt*Abolt=4.038 \text{ kips}$ ok

φVn= $φ*Fnv*Abolt=2.423 \text{ kips}$ ok

Combined Check:

Fv= $Vu/Abolt=0.945 \text{ ksi}$

F'nt= $1.3Fnt-Fnt*Fv/φFnv≤Fnt=49 \text{ ksi}$

φTn= $φ*F'nt*Abolt=4.038 \text{ kips}$ ok

Check 0.75thk Aluminum Plate (LRFD):

$f_y = 11 \text{ ksi}$

φ= 0.9

arm=1 in

b=22 in

M at Plate=S=φMn=

Ratio check=(M at Plate)/(φMn)=0.827 <1 OK

T=11\*Fu=25.319 k

t=0.750 in

T \* arm=25.319 k-in

bt^2/6=2.063 in^3

1.5\*φ\*fy\*S=30.628 k-in

(See Page #2)



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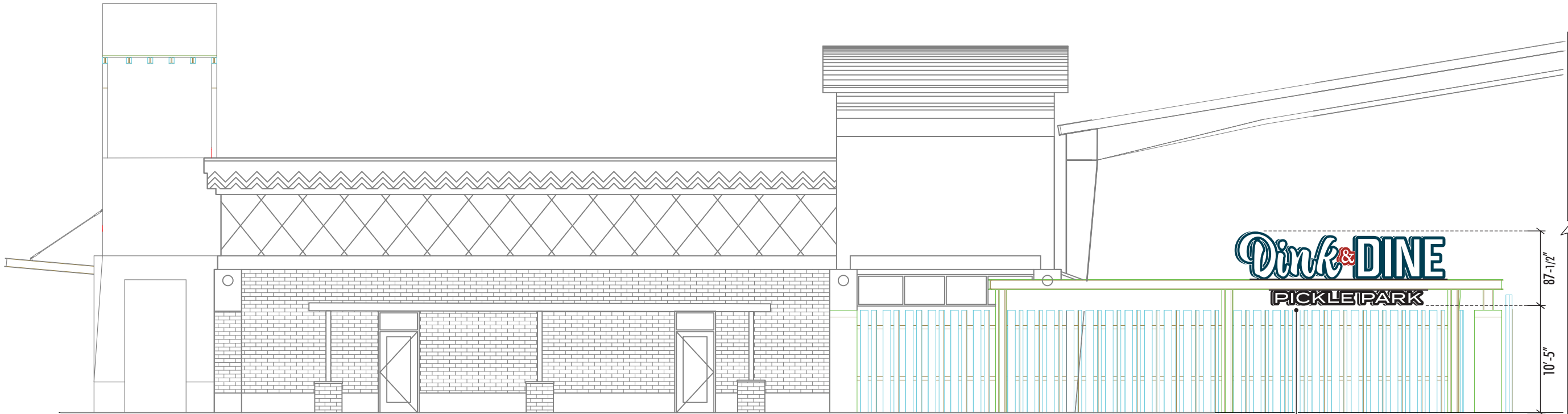
Sign Type: East Tenant Logo - Calcs

Sheet No: ETL.11 18 of 19

proposed channel letter location  
(steel beam support not shown)



**A** PHOTO – CURRENT CONDITIONS  
SCALE: NTS



**CE** CONTEXT ELEVATION, PARTIAL EAST ELEVATION  
SCALE: 3/32" = 1' - 0"

ETL 101



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Rev. Date: **04-30-25**

Sign Type: **East Tenant Logo**

Sheet No: **ETL.20** 19 of 19